



SIZING OPTIMIZATION OF PARAMETRICALLY DESIGNED TRUSSES

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Summary: *In this paper parametric modeling of sizing optimization truss models is developed. Sizing optimization of trusses views element cross-sections as variables with the goal of minimizing overall mass while maintaining equivalent stresses within acceptable ranges, as well as limiting displacement. In order to conduct such a process, models need to be created with parameters, and outputs which can be used to create an objective function. Furthermore the models in each iteration of the optimization are subjected to finite element analyses to determine stress. Parametric models of standard 10 bar, 17 bar, and 25 bar trusses are created to facilitate optimization. The heuristic optimization method used is genetic algorithm. Optimization results obtained from these models are compared to those from literature and the initial model.*

Key words: truss, sizing optimization, parametric model, genetic algorithm

1. INTRODUCTION

Structural truss optimization is an interesting topic of research in the fields of mechanical, civil, and structural engineering. The structural optimization problem determines the best design for a specified problem subjected to certain restrictions. This complex process is very beneficial, as it can lead to lighter and more inexpensive structures, while maintaining structural integrity, through optimizing various construction parameters. The most frequently used optimization types are sizing, shape, topography, topometry, and topology. Sizing optimization views cross section geometries as variables. Most papers published in recent years on the subject of truss sizing optimization use new methods or their variations and use standard test examples of 10, 15, 17, 25, 47, 52, 72, 117, etc. bar trusses. The most commonly used are heuristic optimization methods for their favorable characteristics.

Hasancebi and Azad [1] created and tested a new meta-heuristic method called

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adaptive dimensional search which updates search dimensional parameters in every iteration. They investigated the capabilities and potentials of ADS in structural optimization, and tested their method on various standard test examples of trusses. Cheng et al. [2] tested their new hybrid harmony search algorithm on six test problems achieving very competitive results. Degertekin et al. [3] applied TLBO algorithm to optimize truss structure sizing and compared their results to other meta-heuristic method results. Kaveh et al. [4] used a combination of swarm intelligence and chaos theory to find optimal truss structure cross sections. Kazemzadeh et al. [5] approached the discrete sizing optimization of steel trusses problem using guided stochastic search (GSS) as a design-driven heuristic approach and tested it on 10, 117, 130, 392, and 354 member truss structures. Sizing optimization done by Mortazavi and Toğan [6] showed the method of hyperspheres and showed promising results in using this method for truss optimization. Farshi and Alinia-ziazi [7] included an analysis step in the optimization cycle excluding the need for separate structural analyses and showed the improvement on 10, 22, 25, 72, 60, 132 and 200 bar trusses. Asl et al. [8] gave a detailed description of the sizing optimization problem and an extensive comparison to various other results from literature for the same test problems. Soek et al. [9] showed the importance of the selection of starting values for sizing optimization using harmony search method.

In this paper sizing optimization of standard 10 bar, 17 bar, and 25 bar trusses is conducted on parametric models created for these purposes. Genetic algorithm optimization results are compared to those from literature. The parametric models can be adapted for any truss design sizing optimization due to their parametric nature. The goal is to have a single parametric model which can be used for any design and loading case by simple parametric input changes.

2. PROBLEM FORMULATION

Sizing optimization views cross section parameters as variables and requires an initial model of the truss. These parameters can be cross section geometry (shape) and/or dimensions. Standard test examples optimized in this paper all have circular cross section profiles, and only vary the diameters of each truss element cross section. As cross section diameters are variables the models need to be created with this in mind. The parametric models and optimization in this research are all done in Rhinoceros 5.0 using Grasshopper, Galapagos optimization, and Karamba plugins.

2.1 OPTIMIZATION

For the purposes of this research the heuristic optimization method, genetic algorithm was used due to its favorable characteristics. Genetic algorithm (GA) is a heuristic method for optimizing whose operation is based on mimicking natural/evolutionary processes [10]. The algorithm contains three basic operators: selection, crossover, and mutation. The process of transferring genetic information through generations is called selection. Crossover represents the process/operations between two parents, where an exchange of genetic information and new generations are made. A random change in the genetic structure of some individuals for overcoming early convergence is created by the mutation operator. Algorithm operation is based on survival of the fittest individuals through evolution which exchange genetic material. Selection ranks individuals in the population using values from the fitness function, which

defines the ability/quality of the individual.

Genetic algorithm, due to its convergence characteristics has a widespread application. Researchers are inspired to use this algorithm for scientific purposes, industrial application, business applications and to further increase its use.

In order to properly constrain the complex mathematical model, so that the result of the optimization gives a realistic model, many design factors need to be considered. As the examples tested in this paper are of standard examples taken from literature which have been determined to give applicable resulting models by only considering node displacement and/or stress depending on the example.

In the case of sizing optimization the minimum weight design problem for the truss structures can be defined as:

$$\left\{ \begin{array}{l} \min W(A) = \sum_{i=1}^{i=n} \rho_i A_i l_i \text{ with } A = (A_1, \dots, A_n) \\ \text{subjected to } \begin{cases} A_{\min} \leq A_i \leq A_{\max} & \text{for } i = 1, \dots, n \\ \sigma_{\min} \leq \sigma_i \leq \sigma_{\max} & \text{for } i = 1, \dots, n \\ u_{\min} \leq u_j \leq u_{\max} & \text{for } j = 1, \dots, k \end{cases} \end{array} \right. \quad (1)$$

where n is the number of truss elements, k is the number of nodes, l_i is the length of the i^{th} element, A_i is the area of the i^{th} element cross section, σ_i is the stress of the i^{th} element, u_j is displacement of the j^{th} node.

As cross section area is a variable parameter in the optimization of the trusses, adequate parametric models need to be created to accommodate the optimization method needs. Galapagos optimization for Grasshopper does not have a penalty function input. This has been overcome by having the constraints set to multiply all separate masses of elements with a large number in each generation where any or all of the constraints are not met to ensure that such a result does not end up being a local minimum. The optimization parameters for Galapagos are a maximum stagnant of 50, population of 50, initial boost of 2x, maintaining 5% and +75% inbreeding for the evolutionary solver.

2.2 PARAMETRIC MODELS

For each of the three test examples parametric models were created in Rhinoceros 5. The only variable parameters connected to the optimization operator are the cross sections of elements. The initial model beams are created by parametric input of points and the selection of which points connect to each other. Circular cross sections for all models are initially set to 200mm diameter. Forces and supports are then tied to created points and set as needed by inputting values.

For the 10 bar truss the initial model bar and node layout is given in figure 1. This cantilever truss has 10 independent variables. The material of the truss elements is Aluminum 6063-T5 whose characteristics are: Young modulus 6894.7kN/cm² and specific weight 27.1447kN/cm³. Point loads P_1 are 444.82kN, $P_2=0$ kN in the first load case, and P_1 are 667.233kN, $P_2=222.411$ kN in the second load case as shown in figure 1. The model is limited to a maximal displacement of ± 0.0508 m of all nodes in all directions, axial stress of ± 172.3689 MPa for all bars, and minimum cross-sectional area of all members is limited to 9.045mm diameter.

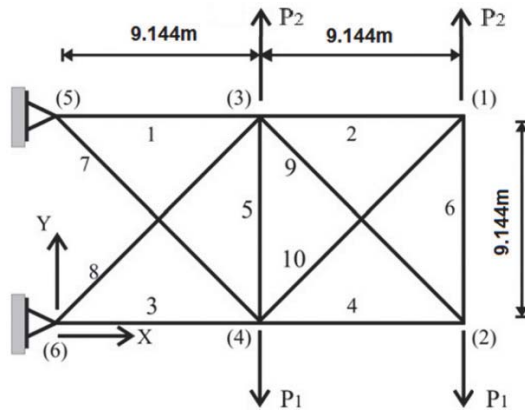


Fig. 1 Initial 10 bar truss model

For the 17 bar truss the initial model bar and node layout is given in figure 2. For this example the material characteristics are: Young modulus 20684.2719kN/cm^2 , shear modulus 8076kN/cm^2 , and specific weight 72.6985kN/cm^3 . A single point load of 444.82kN is applied in node 9, as shown in figure 2. Each bar cross section is an independent variable limited to a minimal cross-sectional area of all members is limited to 64.516mm^2 (9.045mm diameter) as a lower limit to all boundaries. The only constraint is a displacement limitation for all nodes of $\pm 0.0508\text{m}$ of all nodes in both x and y directions.

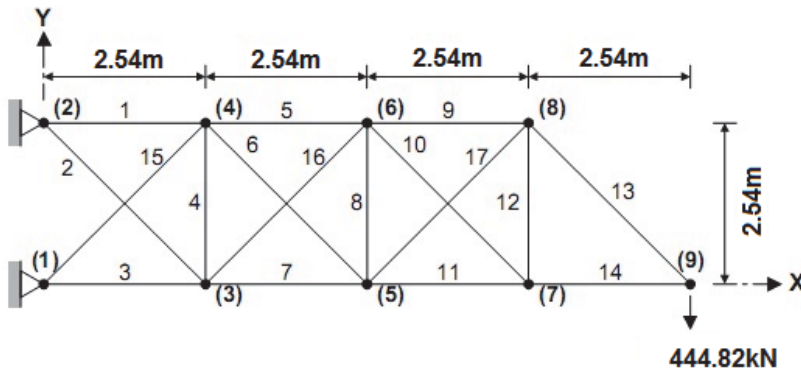


Fig. 2 Initial 17 bar truss model

For the 25 bar truss the initial model bar and node layout is given in figure 3. The material of the truss elements is Aluminum 6063-T5, the same as for the 10 bar truss. This example has two load cases which are given in table 1. This space truss has members cross sections grouped as follows: 1 (A_1), 2 ($A_2 - A_5$), 3 ($A_6 - A_9$), 4 ($A_{10} - A_{11}$), 5 ($A_{12} - A_{13}$), 6 ($A_{14} - A_{17}$), 7 ($A_{18} - A_{21}$), 8 ($A_{22} - A_{25}$). The model is limited to a maximal displacement of $\pm 0.00889\text{m}$ of all nodes in all directions, member stress limitations for bar groups are given in table 2, and minimum cross-sectional area of all members is limited to 64.516mm^2 (9.045mm diameter).

Table 1 25 bar truss example load conditions

Node	Load condition 1 components	Load condition 2 components
	P_x, P_y, P_z [kN]	P_x, P_y, P_z [kN]
1	0, 20, -5	1, 10, -5
2	0, -20, -5	0, 10, -5
3	0, 0, 0	0.5, 0, 0
6	0, 0, 0	0.5, 0, 0

Table 2 Member stress limitation for the 25 bar truss

Member groups	Compressive stress limitation [kN]	Tensile stress limit [kN]
1 (A_1)	241.951	40
2 ($A_2 - A_5$)	79.9102	40
3 ($A_6 - A_9$)	119.314	40
4 ($A_{10} - A_{11}$)	241.951	40
5 ($A_{12} - A_{13}$)	241.951	40
6 ($A_{14} - A_{17}$)	46.6017	40
7 ($A_{18} - A_{21}$)	47.9806	40
8 ($A_{22} - A_{25}$)	76.4077	40

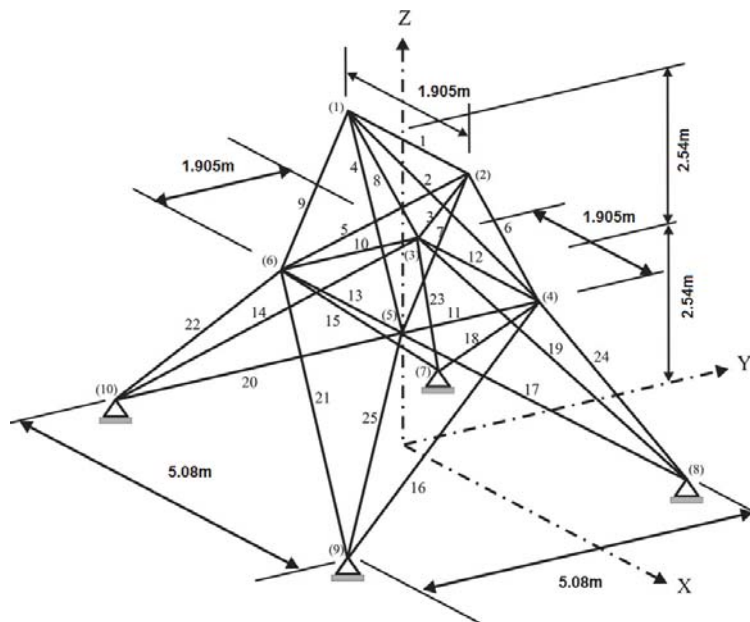


Fig. 3 Initial 25 bar truss model

Models are created in Rhino initially as curves which are collected in Grasshopper and converted to beam elements. Assume these elements do not vary in length the only variable input in their assembly is the cross section diameter for each element individually. Individual weights of elements are calculated from individual cross section areas and multiplied by their length, and specific weight. Once the model is analysed using first order theory of small deflections the model deformation resulting bar compressive and tensile forces are calculated via further operators. Stress is calculated for each of the elements for all load case and compared to stress limits giving either a true or false readout discarding any combination without all elements meeting required

constraints. At the same time displacement is also compared to the limit value. If either of the two conditions are not met each elements weight is multiplied by a large number to ensure that such a combination is not considered a global or local minimum before entering the objective function.

3. RESULTS

The initial weight of the 10 bar truss when all elements have 20 mm thick elements is 9089.765 kg with a maximum displacement of 21.239 mm. Table 3 compares element diameters and total weight of trusses according to loading case. Figure 4 shows the resulting deformed models for both load cases.

Table 3 10 bar truss optimal design comparison by load case

Element number:	GA		[7]		[8]	
	LC1 Φ [mm]	LC2 Φ [mm]	LC1 Φ [mm]	LC2 Φ [mm]	LC1 Φ [mm]	LC2 Φ [mm]
1	163.23	143.23	158.014	138.731	157.9815	138.1159
2	9.04	9.05	9.045	9.045	9.04465	9.04465
3	149.96	142.64	137.764	143.847	137.7651	144.655
4	119.28	122.485	111.594	108.445	111.4828	109.5262
5	9.045	9.05	9.045	9.045	9.04465	9.04465
6	9.045	40.432	21.239	40.142	21.32505	40.13928
7	76.28	103.063	78.1051	100.734	78.12607	100.1688
8	126.83	106.554	131.183	102.426	131.292	101.7163
9	130.3	126.247	132.708	128.963	132.5376	129.0003
10	09.045	9.05	9.045	9.045	2.86017	9.04465
Weight:	2314.1636kg	2171.615kg	2295.81kg	2121.81kg	2294.568	2120.738

The initial weight of the 17 bar truss when all elements have 20mm thick elements is 10044.4355kg with a maximum displacement of 8.887mm. Table 4 compares element diameters and total weight of trusses. Figure 5 shows the resulting deformed model with compressed elements shown in red, and stretched elements in blue.

Table 4 17 bar truss optimal design comparison

Element number:	GA Φ [mm]	[9] Φ [mm]	[8] Φ [mm]
1	109.419	113.765	113.757
2	43.878	9.399	9.272
3	102.403	99.063	99.181
4	9.045	9.045	9.045
5	94.131	81.653	81.468
6	57.046	67.120	67.271
7	83.253	98.371	98.429
8	13.546	9.045	9.045
9	68.956	80.563	80.675
10	9.047	9.045	9.045
11	65.848	57.865	57.710
12	34.799	09.045	9.045
13	68.977	68.046	68.190
14	57.794	57.638	57.266
15	61.124	68.022	67.593
16	49.697	09.045	9.045
17	61.903	67.575	67.565
Weight:	1207.407kg	1170.63477kg	1169.705kg

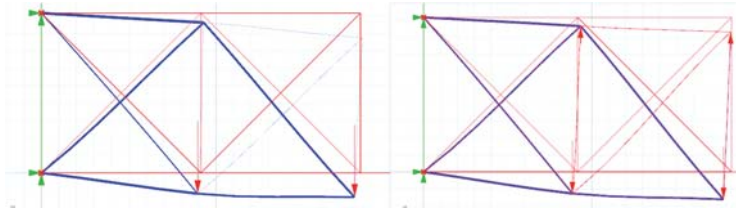


Fig. 4 Deformed optimal model of 10 bar trusses for LC1 (left) and LC2 (right)

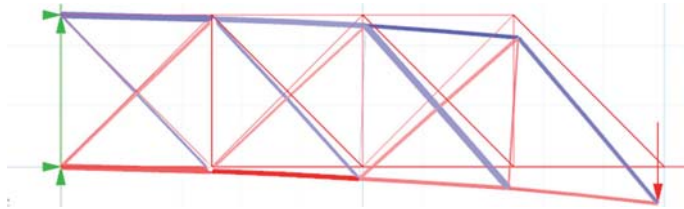


Fig. 5 Deformed optimal model of 10 bar truss

The initial weight of the 25 bar truss when all elements have 20mm thick elements is 7389.295kg with a maximum displacement of 0.402mm. Table 5 compares element diameters and total weight of trusses. Figure 6 shows the resulting deformed model with compressed elements shown in red, and stretched elements in blue.

Table 5 25 bar truss optimal design comparison

Element group:	GA Φ [mm]	[7] Φ [mm]	[8] Φ [mm]
1 (A_1)	2.86	40.4297	2.860
2 ($A_2 - A_5$)	32.02	49.397	40.311
3 ($A_6 - A_9$)	56.36	2.860	49.519
4 ($A_{10} - A_{11}$)	2.86	2.860	2.860
5 ($A_{12} - A_{13}$)	2.86	23.650	2.860
6 ($A_{14} - A_{17}$)	25.97	37.017	23.596
7 ($A_{18} - A_{21}$)	43.3	46.708	36.999
8 ($A_{22} - A_{25}$)	41.85	40.430	46.681
Weight:	261.296kg	247.375kg	247.1532kg

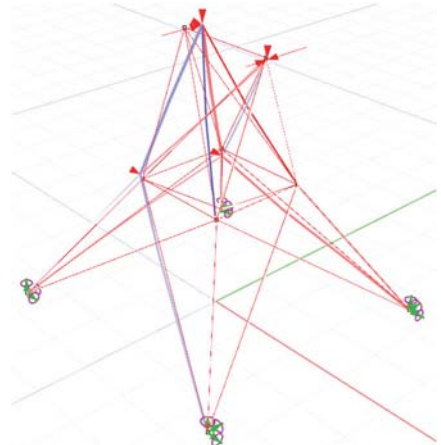


Fig. 6 Deformed optimal model of 25 bar truss

4. CONCLUSION

As most heuristic optimization methods show drastic decreases in weight of standard test models, the benefits of their use in designing practically applicable trusses is obvious. The quickest way to create truss models is through parametric input. By combining parametric modeling with optimization the time needed to achieve a minimal weight design concept is decreased. In this paper through the use of Rhino, a program with integrated modeling, parameterization, finite element analyses, and optimization this process is further integrated to not require additional steps. The parametric models optimization capabilities are proven in this paper by testing the created model on 10, 17, and 25 bar truss models showing a 74.25% (76.11% for Load case 2), 86.72%, and 96.46% decrease in weight respectively. While the weight results vary from those in literature, due to the model using basic genetic algorithm without penalty functions, they are still competitive. The next step in this research will be creating a parametric model with dynamic input of nodes and their connections to optimize shape and topology as well.

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