

Proceedings

**The 6th International Congress
of Serbian Society of Mechanics**

Tara, June 19-21, 2017

Edited by:

**Mihailo Lazarević
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The 6th International Congress of Serbian Society of Mechanics

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Table of Contents

Technical Program	1
List of Contributions	6
General Mechanics (G)	6
Mechanics of Solid Bodies (S)	7
Fluid Mechanics (F)	11
Control and Robotics (C)	11
Interdisciplinary Areas (I)	13
Mini-symposia – Nonlinear Dynamics (M1)	14
Mini-symposia – Turbulence (M2)	15
Mini-symposia – Bioengineering (M3)	17
Plenary Lectures	18
P-1 Locomotion of mobile robotic systems: dynamics and optimization by F. Chernousko	18
P-2 Randomly excited nonlinear dynamic systems endowed with fractional derivatives elements by P. Spanos	35
P-3 Ut Vis Sic Tensio by G. Saccomandi	42
P-4 Design of the antiturbulence surface for producing maximum drag reduction effect by J. Jovanović	56
P-5 Personalized site-specific models of Atherosclerotic plaque progression by T. P. Exarchos et al.	70
P-6 Identification of dynamic properties of mechanical structure from measured vibration responses by Valentina Golubović-Bugarški et al.	80
P-7 Dynamical behavior of composite materials reinforced with strong fibers by D. Milosavljević	102
Award „Rastko Stojanovic”	
RSA Candidate	124
RSA Candidate	126
Abstracts	128
General Mechanics (G)	128
Mechanics of Solid Bodies (S)	133
Fluid Mechanics (F)	144
Control and Robotics (C)	145
Interdisciplinary and Multidisciplinary Problems (I)	149
Mini-symposia – Nonlinear Dynamics (M1)	153
Mini-symposia – Turbulence (M2)	157
Mini-symposia – Bioengineering (M3)	161

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Damir Madjarević
Ines Grozdanović
Nemanja Zorić
Aleksandar Tomović

Foreward

The present volume contains plenary lectures, abstracts and papers of young authors competing for the „Rastko Stojanović” award at the 6th International Congress of Serbian Society of Mechanics. The objectives of this Congress, to be held at Mountain Tara during the period 19th -21th June 2017, are to review and discuss some of the latest trends in various fields of theoretical and applied mechanics as well as it aims to bring together the scientific communities of theoretical and applied mechanics in an effort to facilitate the exchange of ideas on topics of mutual interests, and to serve as a platform for establishing links between research groups with complementary activities.

We are happy to report that the number of accepted papers to be presented at the 6th Congress is 99. In addition, among them, 7 invited plenary lectures were presented by the authors from Russia, USA, Greece, Germany, Serbia and BIH, Republika Srpska. These papers were grouped in the following sections General Mechanics, Fluid Mechanics, Mechanics of Solid Bodies, Control and Robotics, and Interdisciplinary and Multidisciplinary Areas. Also, the three Minisymposia were organized with following topics: Nonlinear Dynamics, Turbulence and Bioengineering.

The Editors would like to express their thanks to all participants for their scientific contribution to 6th Congress of Mechanics, as well as colleagues and friends who helped with the organization. Next, to the distinguished invited lecturers who kindly accepted the invitation to come to Congress and helped make it success. We owe great thanks to the reviewers of the papers, to the members of the Scientific and Organizing Committee, and to the organizers of the Mini-symposia on Nonlinear Dynamics, Turbulence and Bioengineering. The support of the members of Steering Committee of Serbian Society of Mechanics in organizing this event is also appreciated. Finally, special thanks are also due to those organizations which supported financially this Congress: Serbian Society of Mechanics, Ministry of Education, Science and Technological Development of the Republic of Serbia, and Faculty of Mechanical Engineering, University of Belgrade, Belgrade. It is our great pleasure to have you with us at the 6th Congress International Congress of Serbian Society of Mechanics.

We would like to wish all participants of this Congress a warm welcome to our country, our Serbian Society of Mechanics and Venue Congress place at Hotel Omorika, Tara, Serbia.

Tara, June, 2017

The Editors

Mihailo P. Lazarević

Damir Madjarević

Ines Grozdanović

Nemanja Zorić

Aleksandar Tomović

Technical program

SUNDAY, June 18, 2017

19:00 *Welcoming Coctail* (*Banquet hall*)

MONDAY, June 19, 2017

8:00 - 9:00 *Registration of participants* (*Main Hall*)

9:00 - 9:15 Welcome address: (*Forum Palace Hall*)
Professor M. P. Lazarević, the President of
Serbian Society of Mechanics

Plenary Lectures (*Forum Palace Hall*)
Chairman: Mihailo P. Lazarević

9:15 - 10:00 P-1 F. Chernousko
LOCOMOTION OF MOBILE ROBOTIC SYSTEMS: DYNAMICS
AND OPTIMIZATION

10:00 - 10:45 P-2 P. Spanos
RANDOMLY EXCITED NONLINEAR DYNAMIC SYSTEMS
ENDOWED WITH FRACTIONAL DERIVATIVES ELEMENTS

10:45 - 11:10 *Coffee Break* (*Main Hall*)

11:10 - 12:30 Parallel Sessions

Session	G1	S1	C1	F1
Hall	<i>Forum Palace</i>	<i>Banquet hall</i>	<i>Idea hall</i>	<i>Dialogue hall</i>
11:10	G1a	S1a	C1a	F1a
11:30	G1b	S1b	C1b	F1b
11:50	G1c	S1c	C1c	F1c
12:10	G1d	S1d	C1d	F1d

12:10 - 13:20 *Lunch* (*Restaurant*)

Plenary Lecture (*Forum Palace Hall*)

Chairman: Srboľjub Simić

13:30 - 14:15 P-3 G. Saccomandi
UT VIS SIC TENSIO

14:20 -17:30 *Social program (excursion to Mokra Gora and train ride "Sargan Eight")*

17:40 - 19:20 Parallel Sessions

Session	G2	S2	M1	C2
Hall	<i>Forum Palace</i>	<i>Banquet hall</i>	<i>Idea hall</i>	<i>Dialogue hall</i>
17:40	G2a	S2a	M1a	C2a
18:00	G2b	S2b	M1b	C2b
18:20	G2c	S2c	M1c	C2c
18:40	G2d	S2d	M1d	C2d
19:00	G2e	S2e	M1e	

TUESDAY, June 20, 2017Plenary Lectures (*Forum Palace Hall*)

Chairman: G. Saccomandi

9:00 - 9:45 P-4 J. Jovanović

DESIGN OF THE ANTITURBULENCE SURFACE FOR
PRODUCING MAXIMUM DRAG REDUCTION EFFECT

9:45 - 10:30 P-5 T. P. Exarchos et al.

PERSONALIZED SITE-SPECIFIC MODELS OF
ATHEROSCLEROTIC PLAQUE PROGRESSION10:30 - 11:00 *Coffee Break* (*Main Hall*)

11:00 - 13:00 Parallel Sessions

Session	G3	M2	S3	M1
Hall	<i>Forum Palace</i>	<i>Banquet hall</i>	<i>Idea hall</i>	<i>Dialogue hall</i>
11:00	G3a	M2a	S3a	M1f
11:20	G3b	M2b	S3b	M1g
11:40	G3c	M2c	S3c	M1h
12:00	G3d	M2d	S3d	M1i
12:20	G3e	M2e	S3e	M1j
12:40	G3f	M2f	S3f	M1k

13:00 - 14:20 *Lunch* (*Restaurant*)Plenary Lecture (*Forum Palace Hall*)

Chairman: T. P. Exarchos

14:30 - 15:15 P-06 Valentina Golubović-Bugarski et al.

IDENTIFICATION OF DYNAMIC PROPERTIES OF
MECHANICAL STRUCTURE FROM MEASURED VIBRATION
RESPONSES15:15 - 15:40 *Coffee Break* (*Main Hall*)

15:40 - 17:20 Parallel Sessions

Session	S4	M2	C3	I1
Hall	<i>Forum Palace</i>	<i>Banquet hall</i>	<i>Idea hall</i>	<i>Dialogue hall</i>
15:40	S4a	M2g	C3a	I1a
16:00	S4b	M2h	C3b	I1b
16:20	S4c	M2i	C3c	I1c
16:40	S4d		C3d	I1d
17:00	S4e			I1e

17:20 - 18:00 Round table: CURRICULUM IN ENGINEERING MECHANICS
-modernization of teaching mechanics in higher education
(*Forum Palace Hall*)

18:00 - 19:00 General Assembly Meeting of Serbian Society of Mechanics
(*Forum Palace Hall*)

19:00 - 21:30 *Gala Dinner* (*Restaurant Hotel Omorika*)

WEDNESDAY, June 21, 2017Plenary Lecture (*Forum Palace Hall*)

Chairman: V. Golubović-Bugarski

9:00 - 9:45 P-7 D. Milosavljević

DYNAMICAL BEHAVIOR OF COMPOSITE MATERIALS
REINFORCED
WITH STRONG FIBERS

9:50 - 11:10 Parallel Sessions

Session	M3	S5	I2	
Hall	<i>Banquet hall</i>	<i>Idea hall</i>	<i>Dialogue hall</i>	
9:50	M3a	S5a	I2a	
10:10	M3b	S5b	I2b	
10:30	M3c	S5c	I2c	
10:50	M3d	S5d	I2d	

11:10 - 11:40 *Coffee Break* (*Main Hall*)

11:40 - 12:40 Parallel Sessions

Session	M3	S6		
Hall	<i>Idea hall</i>	<i>Dialogue hall</i>		
11:40	M3e	S6a		
12:00	M3f	S6b		
12:20	M3g	S6c		
12:40		S6d		

13:00 Closing Ceremony (*Dialogue hall*)

List of Contributions

General Mechanics (G)

G1 *Chair: Katica R. (Stevanović) Hedrih*
Co-Chair: Srboľjub Simić

G1a Jovo Jarić, Dragoslav Kuzmanović,
ON LINEAR ANISOTROPIC ELASTICITY DAMAGE TENSOR

G1b Vladimir Dragović, Katarina Kukić,
THE SEPARATION VARIABLES FOR THE GENERALIZED KOWALEVSKI
TOP VIA DISCRIMINANTLY SEPARABLE POLYNOMIALS

G1c: Katica R. (Stevanović) Hedrih
EXTENDED CLASSICAL THEORY OF IMPACTS BY KINEMATICS AND
DYNAMICS OF TWO ROLLING BODIES IN SKEW COLLISION

G1d Nenad M. Grahovac, Miodrag M. Žigić
ENERGY DISSIPATION ANALYSIS OF A COLUMN LIKE STRUCTURE
DURING EARTHQUAKE EXCITATION

G2 *Chair: Vladimir Dragović*
Co-Chair: Miodrag Žigić

G2a: Božidar Jovanović
CONTACT SYMMETRIES AND NOETHER THEOREM FOR TIME-
DEPENDENT HOLONOMIC AND NONHOLONOMIC SYSTEMS

G2b: Ljudmila T. Kudrjavceva, Marko D. Topalović, Milan V. Mićunović
RUTTING PROBLEM FOR RUBBER WHEEL MOTION OVER HMA ASPHALT
CONCRETE PAVEMENT

G2c: Vladimir Dragović, Borislav Gajić, Božidar Jovanović
INTEGRATION OF $SO(n-2)$ AND $SO(n-3)$ SYMMETRIC TOPS

G2d: Radoslav Radulović, Bojan Jeremić, Aleksandar Obradović, Zoran Stokić
GLOBAL MINIMUM TIME FOR THE BRACHISTOCHRONIC MOTION OF A
PARTICLE IN AN ARBITRARY FIELD OF POTENTIAL FORCES

G2e: Aleksandar S. Okuka, Miodrag M. Žigić, Nenad M. Grahovac
ON RHEOLOGICAL BEHAVIOR OF ASPHALT CONCRETES

G3 *Chair: Božidar Jovanović*
 Co-Chair: Nenad Grahovac

G3a: Nenad M. Grahovac, Miodrag M. Žigic, S. Goločorbin-Kon, M. Mikov,
D.T. Spasić
A DESCRIPTION OF METHOTREXATE DISTRIBUTION AND EXCRETION IN
A RAT BY A FRACTIONAL MODEL

G3b: Srdjan Jović
VIBRO-IMPACT SYSTEM BASED ON FORCED OSCILLATIONS OF HEAVY
MASS PARTICLE ALONG A ROUGH CICLOID

G3c: Srđan Kostić, Nebojša Vasović, Kristina Todorović, Dragoslav Kuzmanović
STABILITY ANALYSIS OF STATICALLY INDETERMINATE EARTH SLOPES
USING FINITE ELEMENT METHOD

G3d: Bojan Medjo, Marko Rakin, Nenad Gubelj, Walid Musraty, Andrej Likeb,
Ivana Cvijović-Alagić, Aleksandar Sedmak
FRACTURE MECHANICS ANALYSIS OF HETEROGENEOUS CYLINDRICAL
STRUCTURES USING PIPE-RING NOTCHED BEND SPECIMENS

G3e: Sreten Mastilović
A NOTE ON DAMAGE-FRAGMENTATION TRANSITION OF SLENDER
TAYLOR PROJECTILES: SIZE EFFECT AND SCALING BEHAVIOR

G3f: Antonio Rinaldi, Sreten Mastilović
A REMINDER OF THE KRAJCINOVIC APPROACH TO SCALING OF QUASI-
BRITTLE FRACTURE

Mechanics of Solid Bodies (S)

S1 *Chair: Ratko Pavlović*
 Co-Chair: Dušan Zorica

S1a: Dušan Zorica, Teodor M. Atanacković, Zora Vrcelj, Branislava Novaković
NON-LOCAL AXIALLY LOADED ROD PLACED ON VISCOELASTIC AND
PASTERNAK TYPE FOUNDATION: DYNAMIC STABILITY ANALYSIS

S1b: Milan Cajić, Danilo Karličić, Mihailo Lazarević
COMBINED SUB-HARMONIC RESONANCES OF NANOBEAM ON
FRACTIONAL VISCO-PASTERNAK TYPE FOUNDATION

S1c: Miloš Jočković, Matthias Baitsch, Marija Nefovska-Danilović
FREE VIBRATION ANALYSIS OF CURVED BERNOULLI-EULER BEAM
USING ISOGEOMETRIC APPROACH

S1d: I. Pavlović, R. Pavlović, P. Kozić, G. Janevski
INFLUENCE OF PASTERNAK VISCOELASTIC LAYER ON TIMOSHENKO
BEAMS STOCHASTIC STABILITY

S2 *Chair: Ratko Maretić*
 Co-Chair: Ivan Pavlović

S2a: Ratko Maretić, Sanja Ožvat, Armin Berecki, Valentin Glavardanov
BUCKLING OF CIRCULAR GRAPHENE SHEETS IN AN ELASTIC MEDIUM

S2b: Dragan Rakić, Miroslav Živković, Milan Bojović
IMPLICIT STRESS INTEGRATION OF THE ELASTIC-PLASTIC STRAIN
HARDENING MODEL BASED ON MOHR-COULOMB

S2c: Vladimir Lj. Dunić, Nenad A. Grujović, Radovan B. Slavković, Nenad M.
Busarac, Vukašin R. Slavković
FEM ANALYSIS OF CONCRETE GRAVITY DAM BY DAMAGE PLASTICITY
CONSTITUTIVE MODEL

S2d: Dejan B. Momčilović, Ivana D. Atanasovska
APPLICATION OF GRADIENT ELASTICITY ON CORROSION FATIGUE
DAMAGE ASSESSMENT

S2e: Vladimir Milovanović, Aleksandar Dišić, Nikola Jovanović, Gordana Jovičić,
Miroslav Živković
EXPERIMENTAL STUDY OF DEFORMATION BEHAVIOR AND FATIGUE
LIFE OF S355J2+N STEEL GRADE UNDER CYCLIC LOADING

S3 *Chair: Nina Anđelić*
 Co-Chair: Živojin M. Stamenković

S3a: Slaviša Šalinić, Aleksandar Nikolić
DETERMINATION OF NATURAL FREQUENCIES OF A PLANAR SERIAL
FLEXURE-HINGE MECHANISM USING A NEW PSEUDO-RIGID-BODY
MODEL (PRBM) METHOD

S3b: Nina M. Anđelić, Vesna O. Milošević-Mitić, Taško Đ. Maneski, Đorđe D.
 Đurđević
MINIMUM WEIGHT OF OPEN SECTION THIN-WALLED STRUCTURAL
ELEMENTS UNDER STRESS CONSTRAINT

S3c: Aleksandar Tomović
A NOVEL APPROACH TO THE FREE AXIAL-BENDING VIBRATION
PROBLEM OF INHOMOGENEOUS ELASTIC BEAMS WITH VARIABLE
CROSS-SECTIONAL PROFILES

S3d: Dragan V. Čukanović, Aleksandar B. Radaković, Gordana M. Bogdanović,
 Dragan I. Milosavljević, Vladimir N. Geroski
BENDING ANALYSIS OF FUNCTIONALLY GRADED PLATE ACCORDING
TO HIGH ORDER SHEAR DEFORMATION THEORY

S3e: Nikola Nešić, Milan Cajić, Danilo Karličić
NON-LINEAR PRINCIPAL RESONANCE OF AN ORTHOTROPIC AND
MAGNETOELASTIC RECTANGULAR PLATE OSCILLATING ON
FRACTIONAL VISCOELASTIC LAYER

S3f: Jelena D. Petrović, Živojin M. Stamenković, Miloš M. Kocić, Jasmina B.
 Jovanović-Bogdanović, Milica D. Nikodijević
MHD FLOW AND HEAT TRANSFER IN THE POROUS MEDIUM UNDER THE
INFLUENCE OF AN EXTERNALLY APPLIED MAGNETIC FIELD AND
INDUCED MAGNETIC FIELD

S4 *Chair: Dragan Milosavljević*
 Co-Chair: Gordana Kastratović

S4a Aleksandar R. Savić, Marina M. Aškrabić
CORRELATION BETWEEN DYNAMIC MODULUS OF ELASTICITY AND
COMPRESSIVE STRENGTH for SELF-COMPACTING MORTARs

S4b Marija M. Rafailović, Miroslav M. Živković, Vladimir P. Milovanović, Jelena
 M. Živković
E4 AND MITC4+ SHELL FINITE ELEMENT PERFORMANCE ANALYSIS

S4c Gordana Kastratović, N. Vidanović, Aleksandar Grbović
STRESS INTENSITY FACTOR ASSESSMENT FOR MULTIPLE SURFACE
CRACKS

S4d Milena Rajić, Dragan B. Jovanović, Dragoljub S. Živković
THERMOELASTIC STRESS AND DEFORMATION STATE IN GAS PIPES OF
HOT WATER BOILER UNDER THE STATIONARY AND NON STATIONARY
TEMPERATURE FIELD

S4e Marina Četković
IMPERFECTION SENSITIVITY OF PLATES IN THERMAL ENVIRONMENT

S5 *Chair: Miroslav Živković*
 Co-Chair: Stanko Čorić

S5a Stanko B. Čorić
COMPARATIVE BUCKLING ANALYSIS AND DETERMINATION OF THE
EFFECTIVE LENGTH FACTORS USING TWO NUMERICAL PROCEDURES

S5b Andrija Radović, Nikola Blagojević, Dragoslav Šumarac
STABILITY ANALYSIS OF VERTICAL CUTS

S5c Zoran Perović, Dragoslav Šumarac
ELASTOPLASTIC ANALYSIS OF FRAME STRUCTURES SUBJECTED TO
CYCLIC LOADING

S5d Svetlana M. Kostić, Biljana Deretić-Stojanović
AN EFFICIENT MODEL FOR NONLINEAR ANALYSIS OF CIRCULAR CFT
COLUMNS

S6 *Chair: Gordana Kastratović*
 Co-Chair: Nataša Trišović

S6a: Janko Radovanović, Dragoslav Šumarac, Marija Lazović
CALCULATION OF THE LIMIT LOAD FOR THE STEEL ARC OF THE BIOCE
BRIDGE

S6b: Miroslav S. Marjanović, Dragan M. Kovačević
FREE AND FORCED VIBRATION ANALYSIS OF DELAMINATED
COMPOSITE PLATES OF ARBITRARY SHAPE USING TRIANGULAR
LAYERED FINITE ELEMENTS

S6c: Nataša Trišović, Wei Li, Aleksandar Sedmak, Ana Petrović, Radivoje
Mitrović, Zoran Stokić
ITERATIVE METHODS FOR EIGENSENSITIVITY ANALYSIS - A REVIEW

S6d: Nataša Trišović, Wei Li, Nikola Mladenović, Olivera Jeremić, Ines
Grozdanović, Ana Petrović
EIGENSENSITIVITY AND STRUCTURAL OPTIMIZATION WITH ACCENT
ON THE REPEATED FREQUENCIES

Fluid Mechanics (F)

F1 *Chair: Dragan Jovanović*
Co-Chair: Damir Mađarević

F1a: Saša M. Milanović, Miloš M. Jovanović, Boban D. Nikolić, Živan T. Spasić
SOLID PARTICLES VELOCITY DISTRIBUTION IN PNEUMATIC
TRANSPORT OF GRANULAR MATERIALS IN CHANNELS WITH A NON-
CIRCULAR CROSS SECTION TAKING INTO ACCOUNT SECONDARY FLOW

F1b: Miloš M. Jovanović, Boban D. Nikolić, Saša Milanović, Živan Spasić
FORCED RAYLEIGH BENARD CONVECTION SECONDARY INSTABILITY
IN PRESENCE OF TEMPERATURE MODULATION ON BOTH PLATES

F1c: Srbojlob Simić, Ana Jacinta Soares, and Damir Madjarević MULTI-
TEMPERATURE ZND DETONATION MODEL

F1d: Damir Madjarević, Ana Jacinta Soares, and Srbojlob Simić
NUMERICAL ANALYSIS OF DETONATION WAVES IN MULTI-
TEMPERATURE ZND MODEL

Control and Robotics (C)

C1 *Chair: Dragan T. Spasić*
Co-Chair: Nemanja D. Zorić

C1a: Nemanja D. Zorić, Aleksandar M. Tomović, Miroslav M. Jovanović, Nebojša
S. Lukić, Zoran M. Stokić

EFFECT OF PIEZOELECTRIC FIBER-REINFORCED COMPOSITE (PFRC)
ACTUATOR ORIENTATION ON CONTROLLABILITY OF ANTISYMMETRIC
COMPOSITE PLATES FOR ACTIVE VIBRATION CONTROL

C1b: Jovana Kovačević, Mara Bosnić, Dragan T. Spasić
ON APPROXIMATE ANALYTICAL SOLUTIONS OF EQUATIONS OF
MOTION: TODA OSCILLATORS AND AN OPTIMAL ORBITAL TRANSFER

C1c: Miloš M. Živanović
VELOCITY CONTROL OF A MECHANICAL SYSTEM USING SECOND-
ORDER DECOMPOSITION PRINCIPLE

C1d: Petar D. Mandić, Mihailo P. Lazarević, Zoran Stokić, Tomislav B. Šekara
DYNAMIC MODELLING AND CONTROL DESIGN OF SEVEN DEGREES OF
FREEDOM ROBOTIC ARM

C2 *Chair: Slaviša Šalinić*
Co-Chair: Zoran Perović

C2a: Jelena Vidaković, Mihailo P. Lazarević, Vladimir M. Kvrgić, Maja M. Lutovac
Banduka, Stefan M. Mitrović

CONTROL SYSTEM DESIGN OF SPATIAL DISORIENTATION TRAINER

C2b: Marina Bošković, Slaviša Šalinić, Radovan Bulatović, Goran Miodragović
MULTIOBJECTIVE OPTIMIZATION FOR DYNAMIC BALANCING OF FOUR-
BAR MECHANISM

C2c: Miloš D. Lukić, Mihailo P. Lazarević, Petar B. Petrović
ANALYSIS AND OPTIMIZATION OF UNDERACTUATED FINGER FOR
CMSYSLAB ROBOTIC HAND

C2d: Isak Karabegović, Ermin Husak
BY USING DIGITAL TECHNOLOGY ROBOTIC TECHNOLOGY FOLLOW
THE FOURTH INDUSTRIAL REVOLUTION-INDUSTRY 4.0

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ROLL AND PITCH LINK ANGLES OF THE CFS AND SDT

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Interdisciplinary Areas (I)

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THERMAL AND MAGNETIC EFFECTS ON THE FORCED VIBRATION OF AN ELASTICALLY CONNECTED NONLOCAL ORTHOTROPIC DOUBLE-NANOPLATE SYSTEM

I1b: Miloš M. Kocić, Živojin M. Stamenković, Jelena D. Petrović, Jasmina B. Jovanović-Bogdanović, Milica D. Nikodijević

FLOW AND HEAT TRANSFER OF TWO IMMISCIBLE MICROPOLAR FLUIDS IN THE PRESENCE OF UNIFORM MAGNETIC FIELD

I1c: Vukašin Slavković, Nenad Grujović, Aleksandar Dišić, Andreja Radovanović
INFLUENCE OF ANNEALING AND PRINTING DIRECTIONS ON MECHANICAL PROPERTIES OF PLA SHAPE MEMORY POLYMER PRODUCED BY FUSED DEPOSITION MODELING

I2d: Iva Guranov, Srđan Kostić, Nebojša Vasović
PERIODIC AND AUTOREGRESSIVE MODELS OF GROUNDWATER LEVEL DYNAMICS

I1e: Ivan A. Kostić, Olivera P. Kostić, Zoran A. Stefanović
COMPUTATIONAL 2D ANALYSES OF SEVERAL JET VANE TYPES AIMED FOR THE ROCKET ENGINE THRUST VECTOR CONTROL

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COMPUTATION OF WIND TURBINE BLADE AERODYNAMIC
PERFORMANCES WITH SPECIAL ATTENTION TO VORTEX METHODS

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ANALYTICAL MODELING OF BALLISTIC PERFORATION IN PLUG
FORMATION MODE

I2c: Jelena S. Andrić
NUMERICAL STUDY OF THE EFFECTS OF DRIVING PATTERNS ON
ENERGY FLOW AND FUEL CONSUMPTION IN PARALLEL HYBRID
ELECTRIC VEHICLES

I2d: Kosta Velimirović, Nemanja Velimirović
FLIGHT PERFORMANCE DETERMINATION OF THE TURBOJET AIRCRAFT
ORAO

Mini-symposia - Nonlinear Dynamics (M1)

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PHASE PORTRAIT AND ENERGY SURFACES OF NON-LINEAR SYSTEMS
WITH ONE DEGREE OF FREEDOM: CHARACTERISTIC EXAMPLES

M1b: P. Krasilnikov, R. Amelin
ON THE SATURN'S PRECESSION

M1c: Vladimir S. Korolev, Elena N. Polyakhova, Irina Yu. Pototskaya, Alexey A.
Tikhonov,
PROBLEMS OF MOTION STABILITY OF SOLAR SAIL SPACECRAFT

M1d: Ivana D. Atanasovska, Dejan B. Momcilović
DYNAMICS OF COMPLEX SYSTEMS CONSIST OF DEFORMABLE BODIES
IN CONTACT - THE NEW APPROACH

M1e: J. Simonović
NONLINEAR MODELS OF BONE EXTERNALLY EXCITED CELLULAR
REMODELING

M1_2 Chair: *P. Krasilnikov*
Co-Chair: *J. Simonović*

M1f: Nebojša I. Potkonjak, Ljiljana Z. Kolar-Anić, Maja C. Pagnacco, Slobodan R. Anić,
SOME CONSIDERATION ABOUT VOLTAMMOGRAM AS BIFURCATIONS
DIAGRAM IN ELECTROCHEMICAL OSCILLATORY SYSTEMS

M1g: Jelena P. Maksimović, Kristina Stevanović, Itana Nuša Bubanja, Ljiljana Z. Kolar-Anić, Slobodan R. Anić, Nebojša I. Potkonjak, Maja C. Pagnacco
THE NON-LINEAR BRIGGS-RAUSCHER REACTIONS AS A MEDIUM FOR
INVESTIGATION OF THE CAFFEIC ACID CONCENTRATION AND ITS
POTENTIAL ANTIRADICAL ACTIVITY

M1h: Željko Čupić, Ana Ivanović-Šašić, Vladimir Marković, Ana Stanojević,
Slobodan Anić, Nataša Pejić, Ljiljana Kolar-Anić
BIFURCATION ANALYSIS AS THE METHOD FOR DETERMINATION OF
PARAMETERS THAT APPEAR IN THE MODEL OF CONSIDERED PROCESS

M1i: Ljubinko B. Kevac, Mirjana M. Filipović, Ana M. Djurić
ELASTIC S-TYPE CABLE-SUSPENDED PARALLEL ROBOT IN PRESENCE
OF SECOND MODE

M1j: Danilo Z. Karličić, Milan S. Cajić
NONLINEAR DYNAMIC STABILITY OF A FUNCTIONALLY GRADED
NONLOCAL NANOBEM IN THERMAL ENVIRONMENT BY USING
INCREMENTAL HARMONIC BALANCE METHOD

M1k: Marina Trajković-Milenković, Otto T. Bruhns, Dragoslav Šumarac
NUMERICAL ANALYSIS OF FINITE HYPO-ELASTIC CYCLIC
DEFORMATION WITH SMALL AND MODERATE ROTATIONS

M2 Turbulence

Minisymposia - Turbulence (M2)

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FLOW METER

M2b: Dejan Cvetinović, Rastko Jovanović, Jiří Vejražka, Jaroslav Tihon, Kazuyoshi Nakabe, Kazuya Tatsumi

POSSIBILITY OF VORTEX STRUCTURES CONTROL BY MODULATION OF THE NOZZLE EXIT VELOCITY USING LOW-AMPLITUDE OSCILLATIONS

M2c: Jelena Svorcan, Zorana Trivković, Toni Ivanov

COMPUTATIONAL ANALYSIS OF HORIZONTAL-AXIS WIND TURBINE BY DIFFERENT RANS TURBULENCE MODELS

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MHD STEADY AND UNSTEADY FLOW PAST A CIRCULAR CYLINDER

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REYNOLDS NUMBER INFLUENCE ON INTEGRAL AND STATISTICAL CHARACTERISTICS OF THE TURBULENT SWIRL FLOW IN STRAIGHT CONICAL DIFFUSER

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NEURONAL POPULATIONS INDUCED BY NOISE AND INTERACTION
DELAYS

M3b: Miloš Kojić, Miljan Milosević, Vladimir Simić
CONVECTION–DIFFUSION TRANSPORT MODEL USING COMPOSITE
SMEARED FINITE ELEMENT

M3c: Andjelka N. Hedrih, Katica (Stevanović) Hedrih
RESONANCE AS POTENTIAL MECHANISM FOR HOMOLOG
CHROMOSOMES SEPARATION THROUGH BIOMECHANICAL
OSCILLATORY MODEL OF MITOTIC SPINDLE

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FIELD OF CORRECTION FACTORS FOR SMEARED FINITE ELEMENT

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APPLICATION OF DPD METHOD ON MODELLING SEMICIRCULAR
CANALS

M3f: Velibor Isailović, Igor Saveljić, Dalibor Nikolić, Zarko Milosević, Dusan
Pavlović, Nenad D. Filipović
EYE TRACKING ALGORITHM AND COMPUTATIONAL MODELING IN
PREDICTION OF BENIGN PAROXYSMAL POSITIONAL VERTIGO DISEASE

M3g: Nenad Filipović, Velibor Isailović, Žarko Milosević, Dalibor Nikolić, Igor
Saveljić, Milica Nikolić, Bojana Ćirković-Andjelković, Nikola Jagić, Exarchos
Themis, Dimitris Fotiadis, Gualtiero Pelosi, Oberdan Parodi
COMPUTATIONAL MODELING FOR PLAQUE PROGRESSION AND
FRACTIONAL FLOW RESERVE IN THE CORONARY ARTERIES

the subject of following tests: resonant frequency, ultrasonic pulse velocity and compressive strength of mortar. Dynamic modulus of elasticity was calculated using the first two methods mentioned. Results from resonant frequency measurements were then used for calculating compressive strength of the mixtures through different models developed for ordinary concrete and available in the literature. It was shown that the model proposed by Gardner is closest to describing the behavior of mixtures with different component materials, but would need some corrections in order to be used for these purposes.

S4b Marija M. Rafailović, Miroslav M. Živković, Vladimir P. Milovanović, Jelena M. Živković

E4 AND MITC4+ SHELL FINITE ELEMENT PERFORMANCE ANALYSIS

This paper presents a correction of the 4-node MITC (Mixed Interpolation of Tensorial Components) quadrilateral shell finite element, i.e., MITC4 element [1]. Correction refers to the improvement of membrane behavior, where the components of membrane strain in mid-surface of the shell are assumed using the concept of MITC method. In other words, the mid-surface of 4-node quadrilateral shell finite element is subdivided into four triangular domains, in order to construct the assumed membrane strain field by using the membrane strains obtained from those four triangular subdomains of the shell finite element [2]. The purpose is to reduce membrane locking that happen when the MITC4 shell elements are geometrically distorted in curved geometries, while the element retains good membrane behavior. Numerical results obtained with E4 shell finite element [3], which is implemented in software package PAK, are presented at the end of the paper and compared with those obtained with MITC4+ element. This numerical example shows the main problems of E4 element in the case of very thin shells and curved geometries, which should be significantly reduced by using the abovementioned modified element.

S4c Gordana Kastratović, N. Vidanović, Aleksandar Grbović

STRESS INTENSITY FACTOR ASSESSMENT FOR MULTIPLE SURFACE CRACKS

The aim of this paper was to establish and demonstrate the capacity and performances of simple and easy to use approximate method for mode I stress intensity factors calculations in case of multiple surface cracks. This procedure, which is based on principle of superposition, was applied on a configuration with three coplanar semi-elliptical cracks embedded in three dimensional elastic body, subjected to remote uniaxial tensile loading. All cracks are located in the same plane at same distances, in the middle of the body and the applied stress is perpendicular to the cracks plane. For the verification purposes, the stress intensity factors solutions were obtained by using finite element method (FEM) based computer program. The conducted analysis showed that approximate method is, above all, fast and efficient tool for stress intensity factors assessment even in the case of 3D configurations with MSD. The comparison between results also showed, the significance of accurate calculation of SIFs, in order to provide a better understanding and prediction of 3D multiple cracks propagation.

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E4 AND MITC4+ SHELL FINITE ELEMENT PERFORMANCE ANALYSIS

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Abstract

This paper presents a correction of the 4-node MITC (Mixed Interpolation of Tensorial Components) quadrilateral shell finite element, i.e., MITC4 element [1]. Correction refers to the improvement of membrane behavior, where the components of membrane strain in mid-surface of the shell are assumed using the concept of MITC method. In other words, the mid-surface of 4-node quadrilateral shell finite element is subdivided into four triangular domains, in order to construct the assumed membrane strain field by using the membrane strains obtained from those four triangular subdomains of the shell finite element [2]. The purpose is to reduce membrane locking that happen when the MITC4 shell elements are geometrically distorted in curved geometries, while the element retains good membrane behavior. Numerical results obtained with E4 shell finite element [3], which is implemented in software package PAK, are presented at the end of the paper and compared with those obtained with MITC4+ element. This numerical example shows the main problems of E4 element in the case of very thin shells and curved geometries, which should be significantly reduced by using the abovementioned modified element.

Key words: shell finite element, 4-node element, MITC method, membrane locking, convergence

1. Introduction

Although the shell structures are vastly present in the technical practice, their behavior is not easy to predict. Shell finite elements have been developed during the past decades with the purpose of analysing these structures, and their application in the finite element method is immense [4]. Great number of shell finite elements have been presented so far, based on different theoretical approaches. However, the general shell finite element that is absolutely superior over other shell elements does not exist.

The behavior of shell structures belongs to one of the three asymptotic categories, depending on the geometry of the shell, boundary conditions and applied load, which are: membrane dominated, bending dominated and mixed shell behavior [4-6]. However, the ideal element of a shell should give the correct solution that is independent of the shell geometry, asymptotic

category and its thickness. Of course, such element that has an optimal uniform convergence behavior for any problem, even when it is used in distorted meshes, still has not been formulated.

The greatest obstacle in analysis of the shell structure is the dependence of the behavior of the element to the parameter of thickness, known as membrane and shear locking. The MITC method has been successfully used for the development of the finite elements with reduced locking influence, [1,7-11]. At first, this method has been developed to reduce the shear locking of the MITC4 element [1]. The MITC9 and MITC16 quadrilateral shell finite elements were later developed by Bucleam and Bathe, and both shear and membrane locking of this triangular shell finite elements have been successfully reduced by using this method [10, 11]. Beside that, the MITC method has been successfully used to develop isotropic triangular shell elements, MITC3 and MITC6, by Lee and Bathe [9].

There have been several attempts to reduce membrane locking of 4-node continuum mechanics based shell finite elements. The method of reduced integration [12-14] can greatly alleviate membrane locking as well as shear locking, but the elements do not have proper physical rigid body modes. Also, the method of assumed strain field was applied to the membrane strains in order to reduce membrane locking [15]. That modification gave as a result severely deteriorated in-plane behaviors of 4-node shell elements.

In this paper a new MITC4 element is presented, MITC4+ element, with a modification of membrane strains based upon the MITC method. The problem of shear locking in this shell element is solved with the help of a well known assumed field for transverse shear strain of the original MITC4 shell element [1], whereas a new membrane strain field was proposed for the improvement of membrane behavior. This field is obtained by determining the membrane deformation in triangular domains which divide the mid-surface of the shell element. In this way, one gets an element, which is, in the sense of membrane deformation, less sensitive to the irregularity of the shape and the opportunity to determine more accurately the membrane rigidity.

In the following chapter the formulation of the MITC4+ shell element is presented. The formulation of the originally MITC4 element is given in [1,16]. At the end of this paper, the performances of the shell element E4 are presented using the numerical example.

2. MITC4+ element

In the new 4-node quadrilateral shell finite element, the mid-surface of element is divided into four flat triangular domains, where instead of the membrane strains, which are obtained from the displacement field of the element [1], a new covariant membrane strain field is assumed. The new membrane strain field is obtained by introducing the interpolation of the mentioned triangular domains and determining covariant membrane strains into every triangle separately.

2.1 Defining the position of the center point 5

Figure 1 shows four triangular domains on the mid-surface of the shell finite element. Domains are defined so that their intersection is in point 5, which represents the center in the mid-surface of shell finite element [2].

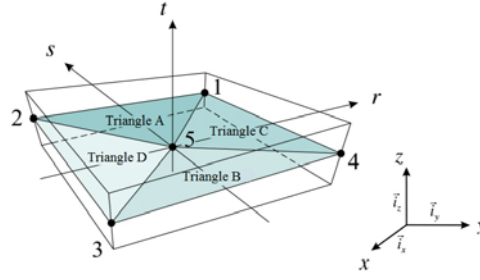


Figure 1. Triangular domains and the central point 5 of the mid-surface shell finite element

The position of central point 5 is given with the following equation

$$X_i^5 = \sum_{k=1}^4 \gamma_k X_i^k \quad i=1,2,3 \quad (1)$$

in which X_i^5 are coordinates of the central point 5, X_i^k are coordinates of nodal point k in global Cartesian coordinate system x, y, z , where $i=1$ corresponds to axis x , $i=2$ to axis y and $i=3$ to axis z .

Similar to equation (1), the displacement of point 5 can be calculated as

$$U_i^5 = \sum_{k=1}^4 \gamma_k U_i^k \quad i=1,2,3 \quad (2)$$

where U_i^5 is the displacement of point 5 in direction of the i axis, while U_i^k is the displacement of node k in the direction of the corresponding axis in the global Cartesian coordinate system x, y, z .

2.2 Interpolation for the geometry and triangular domain displacement

Defined center point is used to divide the mid-surface of the quadrilateral shell finite element into four flat triangular domains, while the geometry interpolation is introduced for every triangular domain in the following form

$$\widehat{x}_i(r,s,t) = \sum_{k=1}^3 \widehat{h}_k(r,s) \widehat{X}_i^k \quad i=1,2,3 \quad (3)$$

where $\widehat{x}_i(r,s,t)$ are the coordinates of the triangular domain's material points, and \widehat{X}_i^k are coordinates of node k which defines the triangular domain A, B, C or D.

Interpolation functions $\widehat{h}_k(r,s)$ of the 3-node triangular elements, that correspond to the nodes that define them are

$$\begin{aligned} \widehat{h}_1(r,s) &= r \\ \widehat{h}_2(r,s) &= s \\ \widehat{h}_3(r,s) &= 1 - r - s \end{aligned} \quad (4)$$

Triangular domains, Figure 1, are defined so that the following is true

$$\begin{aligned} \widehat{\mathbf{X}}^1 &= \mathbf{X}^1, \widehat{\mathbf{X}}^2 = \mathbf{X}^2, \widehat{\mathbf{X}}^3 = \mathbf{X}^5 \quad \text{for triangle A} \\ \widehat{\mathbf{X}}^1 &= \mathbf{X}^3, \widehat{\mathbf{X}}^2 = \mathbf{X}^4, \widehat{\mathbf{X}}^3 = \mathbf{X}^5 \quad \text{for triangle B} \end{aligned} \quad (5)$$

$$\begin{aligned}\widehat{\mathbf{X}}^1 &= \mathbf{X}^4, \widehat{\mathbf{X}}^2 = \mathbf{X}^1, \widehat{\mathbf{X}}^3 = \mathbf{X}^5 \text{ for triangle C} \\ \widehat{\mathbf{X}}^1 &= \mathbf{X}^2, \widehat{\mathbf{X}}^2 = \mathbf{X}^3, \widehat{\mathbf{X}}^3 = \mathbf{X}^5 \text{ for triangle D}\end{aligned}$$

Based on the interpolation function (4) we can define the position of the axes \widehat{r} and \widehat{s} , Figure 2, of the triangular domains.

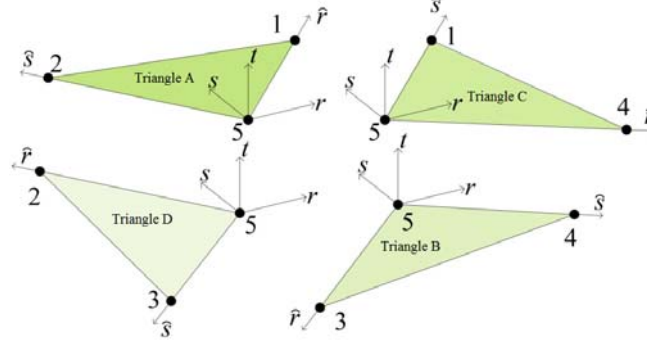


Figure 2. Positions of the triangular domain's axes \widehat{r} and \widehat{s}

Similar to the geometry interpolation, we calculate the displacements of the triangular domains' material points as

$$\widehat{u}_i(r, s, t) = \sum_{k=1}^3 \widehat{h}_k(r, s) \widehat{U}_i^k \quad i=1,2,3 \quad (6)$$

where $\widehat{u}_i(r, s, t)$ are displacements of the triangular domains' material points, and \widehat{U}_i^k the displacement of node k of the corresponding triangular domain in the direction of the i axis.

Displacement vectors of the triangular domains' nodes are defined analogously to (5),

$$\begin{aligned}\widehat{\mathbf{U}}^1 &= \mathbf{U}^1, \widehat{\mathbf{U}}^2 = \mathbf{U}^2, \widehat{\mathbf{U}}^3 = \mathbf{U}^5 \text{ for triangle A} \\ \widehat{\mathbf{U}}^1 &= \mathbf{U}^3, \widehat{\mathbf{U}}^2 = \mathbf{U}^4, \widehat{\mathbf{U}}^3 = \mathbf{U}^5 \text{ for triangle B} \\ \widehat{\mathbf{U}}^1 &= \mathbf{U}^4, \widehat{\mathbf{U}}^2 = \mathbf{U}^1, \widehat{\mathbf{U}}^3 = \mathbf{U}^5 \text{ for triangle C} \\ \widehat{\mathbf{U}}^1 &= \mathbf{U}^2, \widehat{\mathbf{U}}^2 = \mathbf{U}^3, \widehat{\mathbf{U}}^3 = \mathbf{U}^5 \text{ for triangle D}\end{aligned} \quad (7)$$

2.3 Triangular domain's covariant membrane deformations

As the displacements of the material points $\widehat{u}_i(r, s, t)$ of triangular domains are defined by equation (6), the components of covariant membrane strains \widehat{e}_{ij}^m in triangular domains can be calculated with

$$\widehat{e}_{ij}^{m(\Delta)} = \frac{1}{2} \left(\frac{\partial \widehat{\mathbf{x}}}{\partial r_i} \cdot \frac{\partial \widehat{\mathbf{u}}}{\partial r_j} + \frac{\partial \widehat{\mathbf{x}}}{\partial r_j} \cdot \frac{\partial \widehat{\mathbf{u}}}{\partial r_i} \right) = \frac{1}{2} \left(\widehat{\mathbf{g}}_i \cdot \frac{\partial \widehat{\mathbf{u}}}{\partial r_j} + \widehat{\mathbf{g}}_j \cdot \frac{\partial \widehat{\mathbf{u}}}{\partial r_i} \right) \quad i, j=1,2 \quad (8)$$

where $\widehat{\mathbf{g}}_1, \widehat{\mathbf{g}}_2$ and $\widehat{\mathbf{g}}_3 = \mathbf{g}_3$ are covariant base vectors tangent to the coordinate axes of the triangular domains, where the base vector $\widehat{\mathbf{g}}_3$ has the direction of the normal, so it matches the covariant base vector \mathbf{g}_3 of the natural coordinate system r, s, t , Figure 3.

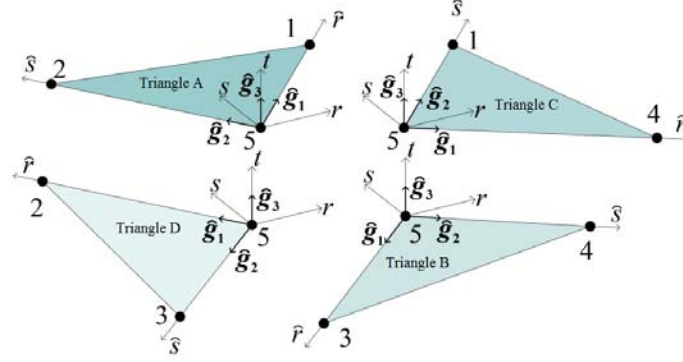


Figure 3. Covariant base vectors of the triangular domains

Deformation vector $\widehat{\mathbf{e}}^{m(\Delta)}$ of the corresponding triangular domain can be represented with a matrix of derivatives of interpolation functions $\widehat{\mathbf{B}}^{(\Delta)}$ and vectors of node displacements \mathbf{U} in the following way

$$\widehat{\mathbf{e}}^{m(\Delta)} = \begin{Bmatrix} \widehat{e}_{11}^{m(\Delta)} \\ \widehat{e}_{22}^{m(\Delta)} \\ \widehat{\gamma}_{12}^{m(\Delta)} \end{Bmatrix} = \widehat{\mathbf{B}}^{(\Delta)} \mathbf{U} = \begin{bmatrix} \widehat{\mathbf{B}}^{1(\Delta)} & \widehat{\mathbf{B}}^{2(\Delta)} & \widehat{\mathbf{B}}^{3(\Delta)} & \widehat{\mathbf{B}}^{4(\Delta)} \end{bmatrix} \begin{Bmatrix} \mathbf{U}^1 \\ \mathbf{U}^2 \\ \mathbf{U}^3 \\ \mathbf{U}^4 \end{Bmatrix} \quad (9)$$

2.4 Strain transformation

When the covariant membrane strains of the triangular domains are completely defined, strain transformations must be done $\widehat{e}_{ij}^{m(\Delta)}$, which are defined by (8), to the natural axes r, s, t of the 4-node shell finite element, of the following form

$$\widehat{e}_{ij}^{m(\Delta)} = \widehat{e}_{kl}^{m(\Delta)} \left(\widehat{\mathbf{g}}_i \cdot \widehat{\mathbf{g}}^k \right) \left(\widehat{\mathbf{g}}_j \cdot \widehat{\mathbf{g}}^l \right), \quad i, j = 1, 2 \quad k, l = 1, 2 \quad (10)$$

where $\widehat{\mathbf{g}}^k$ and $\widehat{\mathbf{g}}^l$ are the covariant vectors of the corresponding triangular domain, while $\widehat{\mathbf{g}}^i \cdot \widehat{\mathbf{g}}_j = \delta_{ij}$ is the Kronecker delta symbol.

Equation (10) in expanded form can be written as

$$\widehat{\mathbf{e}}^{m(\Delta)} = \begin{Bmatrix} \widehat{e}_{11}^{m(\Delta)} \\ \widehat{e}_{22}^{m(\Delta)} \\ \widehat{\gamma}_{12}^{m(\Delta)} \end{Bmatrix} = \begin{bmatrix} \left(\widehat{\mathbf{g}}_1 \cdot \widehat{\mathbf{g}}^1 \right) \left(\widehat{\mathbf{g}}_1 \cdot \widehat{\mathbf{g}}^1 \right) & \left(\widehat{\mathbf{g}}_1 \cdot \widehat{\mathbf{g}}^2 \right) \left(\widehat{\mathbf{g}}_1 \cdot \widehat{\mathbf{g}}^2 \right) & 2 \left(\widehat{\mathbf{g}}_1 \cdot \widehat{\mathbf{g}}^1 \right) \left(\widehat{\mathbf{g}}_1 \cdot \widehat{\mathbf{g}}^2 \right) \\ \left(\widehat{\mathbf{g}}_2 \cdot \widehat{\mathbf{g}}^1 \right) \left(\widehat{\mathbf{g}}_2 \cdot \widehat{\mathbf{g}}^1 \right) & \left(\widehat{\mathbf{g}}_2 \cdot \widehat{\mathbf{g}}^2 \right) \left(\widehat{\mathbf{g}}_2 \cdot \widehat{\mathbf{g}}^2 \right) & 2 \left(\widehat{\mathbf{g}}_2 \cdot \widehat{\mathbf{g}}^1 \right) \left(\widehat{\mathbf{g}}_2 \cdot \widehat{\mathbf{g}}^2 \right) \\ \left(\widehat{\mathbf{g}}_1 \cdot \widehat{\mathbf{g}}^1 \right) \left(\widehat{\mathbf{g}}_2 \cdot \widehat{\mathbf{g}}^1 \right) & \left(\widehat{\mathbf{g}}_1 \cdot \widehat{\mathbf{g}}^2 \right) \left(\widehat{\mathbf{g}}_2 \cdot \widehat{\mathbf{g}}^2 \right) & 2 \left(\widehat{\mathbf{g}}_1 \cdot \widehat{\mathbf{g}}^1 \right) \left(\widehat{\mathbf{g}}_2 \cdot \widehat{\mathbf{g}}^2 \right) \end{bmatrix} \begin{Bmatrix} \widehat{e}_{11}^{m(\Delta)} \\ \widehat{e}_{22}^{m(\Delta)} \\ \widehat{\gamma}_{12}^{m(\Delta)} \end{Bmatrix} = \widehat{\mathbf{T}}^{(\Delta)} \widehat{\mathbf{e}}^{m(\Delta)} \quad (11)$$

where $\widehat{\mathbf{T}}^{(\Delta)}$ is the transformation matrix of the corresponding triangular domain.

Submatrices $\widehat{\mathbf{B}}^{k(\Delta)}$ of the corresponding triangular domain are equal to

$$\bar{\mathbf{B}}^k = \widehat{\mathbf{T}} \widehat{\mathbf{B}}^k \quad (12)$$

Now, the strains of the corresponding triangular domain can be presented like the following, using (9)

$$\bar{\mathbf{e}}^{m(\Delta)} = \widehat{\mathbf{T}}^{(\Delta)} \widehat{\mathbf{B}}^{(\Delta)} \mathbf{U} \quad (13)$$

2.5 Assumed membrane strain field

Using the covariant membrane strains (13) obtained from four triangular domains, a strain field was suggested that reduces the sensitivity to the irregularities of shape, and has the following form

$$\tilde{e}_{ij}^m = a + br + cs + drs \quad i, j = 1, 2 \quad (14)$$

Where the coefficients a, b, c, d are obtained by using the covariant membrane strains from the triangular domains, in the following way

$$a = \frac{1}{4} \left(\bar{e}_{ij}^{m(A)} + \bar{e}_{ij}^{m(B)} + \bar{e}_{ij}^{m(C)} + \bar{e}_{ij}^{m(D)} \right),$$

$$b = \frac{1}{2} \left(-\bar{e}_{ij}^{m(D)} + \bar{e}_{ij}^{m(C)} \right),$$

$$c = \frac{1}{2} \left(-\bar{e}_{ij}^{m(B)} + \bar{e}_{ij}^{m(A)} \right),$$

$$d = 0$$

Assumed strain field (14) is defined so that the positions of points A, B, C and D are symmetrical regarding the element's center, as shown in Figure 4 [2].

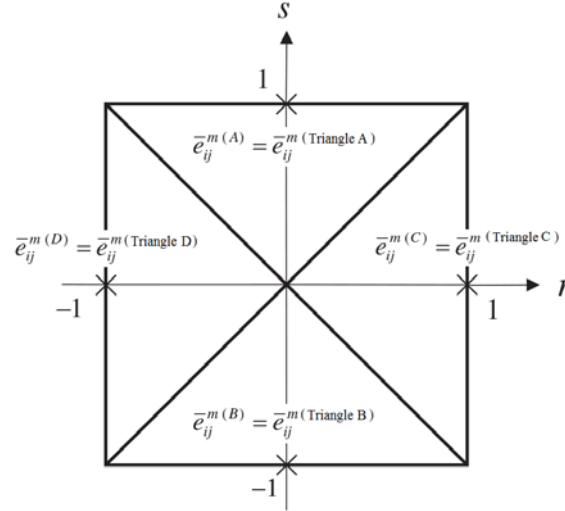


Figure 4. Tying positions of points (A), (B), (C) and (D), that belong to the assumed membrane strain field of the MITC4+ shell element

Considering the transformation (11), and in accordance with (12) and (13), strain vector $\tilde{\mathbf{e}}^m$ of the shell element can be written in following form

$$\begin{aligned} \tilde{\mathbf{e}}^m = & \left\{ \frac{1}{4} \left(\widehat{\mathbf{T}}^{(A)} \widehat{\mathbf{B}}^{(A)} + \widehat{\mathbf{T}}^{(B)} \widehat{\mathbf{B}}^{(B)} + \widehat{\mathbf{T}}^{(C)} \widehat{\mathbf{B}}^{(C)} + \widehat{\mathbf{T}}^{(D)} \widehat{\mathbf{B}}^{(D)} \right) + \left(-\widehat{\mathbf{T}}^{(D)} \widehat{\mathbf{B}}^{(D)} + \widehat{\mathbf{T}}^{(C)} \widehat{\mathbf{B}}^{(C)} \right) \right\}_r + \\ & + \left(-\widehat{\mathbf{T}}^{(B)} \widehat{\mathbf{B}}^{(B)} + \widehat{\mathbf{T}}^{(A)} \widehat{\mathbf{B}}^{(A)} \right) \Big|_s \Big\} \mathbf{U} = \widetilde{\mathbf{B}} \mathbf{U} \end{aligned} \quad (15)$$

where $\widehat{\mathbf{T}}^{(\Delta)}$ are corresponding transformation matrices of the triangular domains, and $\widehat{\mathbf{B}}^{(\Delta)}$ is a matrix of derivatives of interpolation functions of triangular domains defined for every triangular domain. The matrix of derivatives of interpolation functions $\widetilde{\mathbf{B}}$ of the shell element in a natural coordinate system that fits the suggested strain field is obtained from this equation.

3. Numerical analysis

The convergence studies for the E4 shell finite element which is implemented into the software package PAK are presented in this chapter. The obtained results are compared with those of MITC4+ shell element. In order to display improved performances of MITC4+ shell finite element comparing to those of E4 element, the convergence curves of both elements are presented. Convergence curves of MITC4+ element are taken from [2].

For convergence studies, we use the s-norm [17] defined as

$$\|\mathbf{u} - \mathbf{u}_h\|_s^2 = \int_{\Omega} \Delta \boldsymbol{\varepsilon}^T \Delta \boldsymbol{\tau} d\Omega \quad (16)$$

where \mathbf{u} denotes the exact solution and \mathbf{u}_h denotes the solution obtained by finite element discretization. Here, $\boldsymbol{\varepsilon}$ and $\boldsymbol{\tau}$ are the strain vector and the stress vector in the global Cartesian coordinate system, respectively.

In the practical use of this norm, the finite element solution using a very fine mesh \mathbf{u}_{ref} is adopted instead of the exact solution \mathbf{u} , so the equation (16) can be expressed as

$$\|\mathbf{u}_{ref} - \mathbf{u}_h\|_s^2 = \int_{\Omega_{ref}} \Delta \boldsymbol{\varepsilon}^T \Delta \boldsymbol{\tau} d\Omega_{ref} \quad (17)$$

It is well known that the behavior of the element significantly deteriorates with decreasing of the thickness, so the relative error E_h is used for analysis of the sensitivity of the element to the thickness parameter

$$E_h = \frac{\|\mathbf{u}_{ref} - \mathbf{u}_h\|_s^2}{\|\mathbf{u}_{ref}\|_s^2} \quad (18)$$

3.1 Cylindrical shell problems

We observe a cylindrical shell of uniform thickness t , length $2L$ and radius R , which is loaded by the pressure $p(\theta)$ normal to the shell surface, as shown in Figure 5

$$p(\theta) = p_0 \cos(2\theta)$$

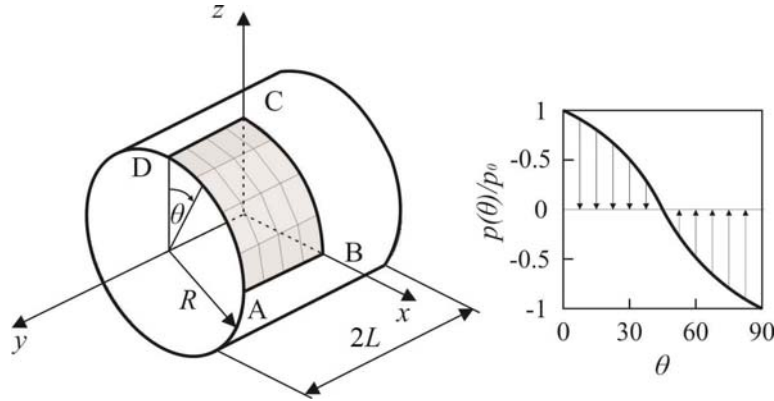


Figure 5. Cylindrical shell loaded by the pressure distribution $p(\theta)$ with free ends

Material properties and dimensions are: $L=R=1\text{m}$, $E=2\times 10^5\text{MPa}$, $\nu=1/3$, $p_0=1\text{Pa}$. A detailed study of this shell problem is presented in [2, 18].

Depending on the boundary conditions at shell structure ends, it shows two different asymptotic behaviors: the bending dominated behavior under a free boundary condition and the membrane dominated behavior under a clamped boundary condition. For the clamped cylindrical shell problems both shell elements, MITC4 and MITC4+, give similar good convergence behavior, as shown in [2], so we consider only a bending dominated behavior.

Because of the symmetry, we can limit calculations to the region ABCD, shown in Figure 5. For the free edge case, the following boundary conditions are imposed: $v = \varphi_1 = 0$ along BC, $u = \varphi_2 = 0$ along DC and $w = \varphi_2 = 0$ along AB. As shown in [2], when using regular mesh all 4-node shell elements have a flat geometry, so the membrane locking does not occur and because of that cylindrical shell was modeled with distorted mesh with $N \times N$ ($N = 4, 8, 16, 32, 64$).

Figure 6 present the convergence curves of the E4 and MITC4+ shell finite elements for the free cylindrical shell problem. Results are obtained for three values of thickness parameter t/L : $1/100$, $1/1000$ and $1/10000$.

The relative error used here is based on the reference solution obtained with a regular mesh of 72×72 E4 shell elements. The element size in the convergence curves is $h = L/N$.

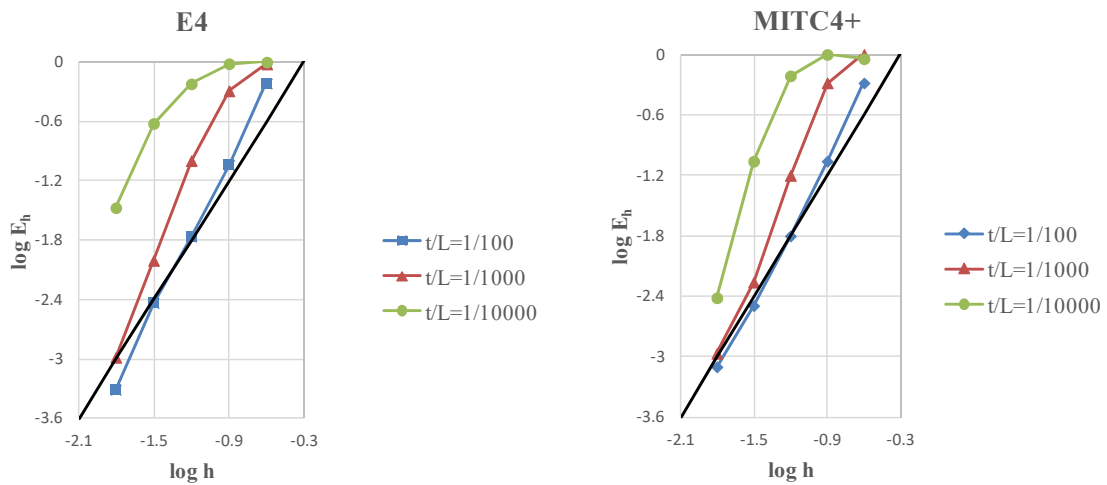


Figure 6. Convergence curves for the free cylindrical shell problem with distorted meshes

Figure 6 shows the main problem in the analysis of shell finite element, that is the influence of thickness parameter of distorted elements on the accuracy of the solution. Very thin shells extremely slowly converge to the reference solution. By increasing mesh density, numerical solution of very thin shells is approaching to the reference solution very slowly with a large deviation. It is obvious that by reducing thickness of distorted shell finite element, the speed of convergence to the reference solution is decreased, i.e. thicker shells for distortion shows better convergence.

It can be seen that MITC4+ shell finite element shows a slightly better convergence in the case of very thin shell. The obtained values are close enough to the exact values comparing to those of E4 shell element.

4. Conclusions

The main idea of this paper is the formulation of the modified 4-node quadrilateral MITC4 shell finite element. With the introduction of the center point 5, four triangular domains are defined. In that way the suggested membrane strain field is constructed using membrane strains obtained from triangular domains, thus eliminating the bilinear term in the field of strain. Theoretical proposals of this element are presented with aforementioned correction of its field with the aim of alleviation of the membrane locking which is strongly present in the distorted elements.

Major problems of the E4 shell element are also presented in this paper by numerical analysis of distribution of loaded pressure $p(\theta)$ on a cylinder with free ends, which shall be significantly reduced with the application of the modified element presented in this paper. The fact that the finite shell elements show variable sensitivity with reducing of thickness, depending on their geometry and boundary conditions, was confirmed with the same example. It has been observed that in distortion elements the influence of the thickness parameter is especially evident, i.e., the speed of convergence of the numerical solution to the correct one is significantly slower in case of reduced thickness. The element shows great sensitivity to the irregularity of shape, which is a very important property for general application, and it must be taken into consideration because it is usually impossible to avoid the finite distortion elements in modeling of structures.

The major problem that occurs during the analysis of finite elements is related to the reduction of the locking which appears in the shells with dominant bending, as in the case of the cylinder with free boundaries analysed in this paper. The tendency in analysis of finite elements of shells is to formulate the element that will converge uniformly, independent of the geometry, boundary conditions and its thickness. Of course, such element still does not exist and it is extremely difficult to formulate it. However, the application of the correction, presented in this paper, should reduce the sensitivity of the element when it is geometrically distorted. It is still necessary to implement the corrected element in the programming package PAK and test its performance on specific examples.

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